

STRUCTURAL DESIGN FOR A TENSTORIED RESIDENTIAL BUILDING WITH ROOFTOP COMMUNITY SPACE AT JATRABARI, DHAKA

## Advisor



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## Contents



Background of the Project


07
Drafting


Objectives of the Study
$\qquad$
Analysis and Design


03
Review of the Documents Supplied by the Client

06
Verification of the Model

## 09 Conclusion

09
Submission Files

## Background of the Project

A 10 Storied Framed RCC Residential Building of approximately 5 katha at Jatrabari, Dhaka including,
$>3$ units in each floor
> A parking space
$>$ A space for drivers in the ground floor
$>$ A fully functioning lift
$>$ A rooftop community space

## Objectives of the Study

$>$ To investigate the site
$>$ To assess environmental impact (EIA)
$>$ To complete the design and analysis of the building with economy, safety, serviceability, and durability
$>$ Reinforcement detailing of the structural components
> Project planning
$>$ To prepare the Bill of Quantity (BOQ)

## Review of the Documents Supplied by the Client

## Super- Structure

> Maximum Ground Coverage (MGC)
> Setback
$>$ Floor Area Ratio (FAR)
> Minimum Occupancy Requirements


## Cont. Review of the Documents Supplied

 by the Client|  | Existing | Required according to Imarat Nirman Bidhimala-2008 | Remarks |
| :---: | :---: | :---: | :---: |
| Maximum Ground Coverage (MGC) | 77.78\% | 62.5\% | Not satisfactory |
| Setback | 1.88 m in south and no setback available in north. | 1.5 meter in front side, 2 meter in the rear side, and 1.25 meter in each side. <br> 4.5 m or 1.5 m from the plot boundary from the center of the existing road. | Not satisfactory |
| Floor area ratio (FAR) | 2600 sq. ft. | 1400 sq. ft. | Not satisfactory |
| Minimum <br> Occupancy Requirements | For unit 1, <br> Area of bedroom 1 and 2 are 31.09 $\mathrm{m}^{2}$ and $31.58 \mathrm{~m}^{2}$. Width of bedroom 1 and 2 are 2.5 m and 2.87 m . Area of bedroom 3 is $29.178 \mathrm{~m}^{2}$, and width is 2.9 m . | For each unit one room with $9.5 \mathrm{~m}^{2}$ area and minimum width of 2.5 m . other rooms, area should be $5 \mathrm{~m}^{2}$ and width should be 2 m | Satisfactory for all 3 units. |

## Review of the Documents Supplied by the Client

## Sub- Structure

> Number and location of boreholes for soil investigation
$>$ Depth of Borehole
> Ground Water Table (GWT)
> Factor of Safety (FS)


## Cont. Review of the Documents Supplied

 by the Client|  | Existing | Required | Remarks |
| :---: | :---: | :---: | :---: |
| Number and location of <br> boreholes for soil <br> investigation | 3 | 5 | Not satisfactory |
| Depth of Borehole- | At least one <br> borehole with a <br> depth of 30 m or <br> 100 ft. | Bore hole-02 has a depth <br> of 30 m | Satisfactory |
| Factor of Safety (FS) | 2.5 | 2.5 | Satisfactory |

## Feasibility Study

## Site Visit



## Cont. Feasibility Study

## Purposes of the Site Visit

To observe-
> Topography of the site
$>$ Proximity of other buildings
> Most economical routes
> Availability of public utility services.


Site Condition

## Cont. Feasibility Study

## Drawbacks of the Site

> Narrow road
> Unavailability of lodging units for workers
> Untidy site


## Cont. Feasibility Study

## Favorable Aspects of the Site

- Easily accessible by rickshaws, private vehicles, pickups and CNGs
> Utility facilities
$>$ Storage space
> Local work force
$>$ Dumping zone


Proposed Storage Space

## Cont. Feasibility Study

## Environmental Impact Assessment

> Air Pollution
> Soil Pollution
> Water Pollution
> Noise Pollution

## Cont. Feasibility Study

## Health, Safety and Societal Impact Assessment

Health and Safety Impacts
> Breathing and Lung Problems
> Electric Shock and Burns
$>$ Hearing Deficiency to the Laborers

## Societal Impacts

> Employment
> Economic Aspect
> Local Use
> Effects on Traffic Volume

## Analysis and Design

Basic Considerations

| Dead Load | Floor Finish (Roof and Floor Slab) | 20 psf |
| :---: | :---: | :---: |
|  | Wall load on beam | $0.51 \mathrm{k} / \mathrm{ft}$. |
|  | Parapet wall load | $0.151 \mathrm{k} / \mathrm{ft}$. |
|  | Partition wall load | 44.7 psf |
|  | Partition wall load (AB-46 Span) | 51.407 psf |
| Live Load | Floor Slab | 41.78 psf. |
|  | Roof Slab (Community Space at Roof) | 100.282 psf |
| Wind Load | Basic wind speed | 210 mph . |
|  | Exposure type | B |
|  | Importance factor | 1 |
|  | Wind pressure for X direction | 1.282 |
|  | Wind pressure for Y direction | 1.572 |
| Earthquake Load | Soil profile type | SD |
|  | Seismic zone factor | 0.15 |
|  | Overstrength factor | 8 |

## Cont. Analysis and Design

## Load Combinations

$>$ Dead load + Live load + Super imposed dead load.
> $1.4 \mathrm{DL}+1.7 \mathrm{LL}$.
$>0.75$ [1.4 DL+1.7 LL+1.7 EQ-x direction].
$>0.75$ [1.4 DL+1.7 LL+1.7 EQ-y direction].
$>0.75$ [1.4 DL + 1.7 LL + 1.7 Wind-x Positive].
$>0.75[1.4$ DL + 1.7 LL + 1.7 Wind-x Negative].
$>0.75$ [1.4 DL + 1.7 LL + 1.7 Wind-y Positive].
$>0.75$ [1.4 DL + 1.7 LL + 1.7 Wind-y Negative].

## Cont. Analysis and Design

## Boundary Condition

> Pile foundation fixed support.
Analysis Software
> ETABS


## Cont. Analysis and Design

## Check for Serviceability

$>$ Serviceability checking includes the storey drift and displacements due to the lateral load.

Displacement check:

| Column |  | Beam |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Corner Column | Side Column | Middle Column | X- Direction | Y-Direction |
| $10 " \times 26 "$ | $10 " \times 28 "$ | $12 " \times 28 "$ | $10 " \times 15 "$ | $10 " \times 18 "$ |

## Cont. Analysis and Design

| Load Combinations | Allowable <br> Deflection as <br> per BNBC <br> (inch) | Maximum <br> deflection at the <br> roof <br> (x-direction) <br> Inch | Maximum <br> deflection at the <br> roof <br> (y-direction) <br> inch | Remarks |
| :---: | :---: | :---: | :---: | :---: |

## Cont. Analysis and Design

Changing the sectional properties:

| Column |  | Beam |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Corner Column | Side Column | Middle Column | X- Direction | Y-Direction |
| $15 " \times 30 "$ | $15 " \times 36 "$ | $12 " \times 28 "$ | $12 " \times 28 "$ | $12 " \times 30 "$ |

## Cont. Analysis and Design

$\left.\begin{array}{|c|c|c|c|c|}\hline \text { Load Combinations } & \begin{array}{c}\text { Allowable } \\ \text { Deflection as per } \\ \text { BNBC } \\ \text { (inch) }\end{array} & \begin{array}{c}\text { Maximum } \\ \text { deflection at the } \\ \text { roof } \\ \text { (x-direction) } \\ \text { inch }\end{array} & \begin{array}{c}\text { Maximum } \\ \text { deflection at the } \\ \text { roof } \\ \text { (y-direction) } \\ \text { inch }\end{array} & \text { Remarks }\end{array}\right\}$

## Cont. Analysis and Design



Deflection graph for S2 (DL+LL+EQ-x)


Deflection graph for S3 (DL+LL+EQ-y)

## Cont. Analysis and Design



Deflection graph for S4 (DL+LL+Wind-x Positive)


Deflection graph for $\mathbf{S 5}$ (DL+LL+Wind-x Negative)

## Cont. Analysis and Design



Deflection graph for S6 (DL+LL+Wind-y Positive)


Deflection graph for S6 (DL+LL+Wind-y Negative)

## Cont. Analysis and Design

## Story Drift Check

| Story | Height | Drift <br> S2 (DL+LL+EQ-x) | Drift <br> (DL+LL+EQ-y) | Allowable <br> Story drift | Remarks |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Roof | 110 | 0.000688 | 0.000487 | 0.1257 | Safe |
| Story 9 | 100 | 0.000764 | 0.000581 | 0.1257 | Safe |
| Story 8 | 90 | 0.000842 | 0.000669 | 0.1257 | Safe |
| Story 7 | 80 | 0.000949 | 0.000752 | 0.1257 | Safe |
| Story 6 | 70 | 0.001122 | 0.000821 | 0.1257 | Safe |
| Story 5 | 60 | 00 | 0.001169 | 0.000869 | 0.1257 |
| Story 4 | 40 | 0.001108 | 0.000888 | 0.1257 | Safe |
| Story 3 | 30 | 0.000924 | 0.000758 | 0.1257 | Safe |
| Story 2 | 20 | 0.001235 | 0.000587 | 0.1257 | Safe |
| Story 1 | 10 | SO | 0.000665 | 0.1257 | Safe |
| GF |  |  | Safe |  |  |

## Verification of the Model

We have provided-
$>$ Reinforcement Area of Beam
> Dead Loads and Live Loads
> SFD and BMD for Dead Load and Live Load
> Lateral Load Analysis

## Cont. Verification of the Model

## Reinforcement Area of Beam

|  | ETABS (in ${ }^{2}$ ) | Hand Calculation <br> ( $\mathrm{in}^{2}$ ) | Comment |
| :---: | :---: | :---: | :---: |
| Left Support (Top) | 1.08 | 1.023 | $5.278 \%$ variation only |
| Left Support (Bottom) | 0.9952 | 1.06 | $6.511 \%$ variation only |
| Right Support (Top) | 1.0454 | 1.06 | $1.397 \%$ variation only |
| Right Support (Bottom) | 1.1 | 0.99 | $10 \%$ variation only |
| Mid Span (Top) | 0.3827 | 0.351 | $8.283 \%$ variation only |
| Mid Span (Bottom) | 0.4146 | 0.38 | $8.345 \%$ variation only |

## Cont. Verification of the Model

## Live Load Beam SFD

| Grid (3-ABDFG) | ETABS <br> $(\mathbf{k})$ | Hand Calculation <br> $(\mathbf{k})$ | Comment |
| :---: | :---: | :---: | :---: |
| 3-AB | 8.624 | 8.44 | $2.18 \%$ variation |
| 3-BD | 5.848 | 6.27 | $6.511 \%$ variation |
| 3-DF | 2.74 | 3.2 | $14.3 \%$ variation |
| 3-FG | 5.938 | 6.178 | $3.88 \%$ variation |

## Lateral Load Analysis

| Grid (A-1346) | ETABS <br> (k) | Hand <br> Calculation <br> (k) | Comment |
| :---: | :---: | :---: | :---: |
| $\mathbf{A - 1 3}$ | 0.999 | 1.08 | $7.5 \%$ variation |

## Drafting

## Beam Column Layout



CORNER COLUMNS = 15"X30"
PERIPHERAL COLUMNS = 15 " X 36
MIDDLE COLUMNS = 12"X28"

## Reinforcement Detailing of Slab



## Cont. Drafting

## Reinforcement Detailing of Stair



## Cont. Drafting

## Reinforcement Detailing of Beam



## Cont. Drafting

## Reinforcement Detailing of Column

| cOLUMNS | CORNER COLUMNS (A1, A6, G2, G7) | COLUMNS | PERIPHERAL COLUMNS (A3, A4, B1, B6, D1, F2, F7, G3, G4) | COLUMNS | MIDDLE COLUMNS (B3, B4, D3, D4, F3, F4) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| BASE TO 1ST FLOOR |  | BASE TO 1ST FLOOR |  | $\begin{aligned} & \text { BASE TO 2ND } \\ & \text { FLOOR } \end{aligned}$ |  |
| $\begin{aligned} & \text { 2ND TO 9TH } \\ & \text { FLOOR } \end{aligned}$ |  | 2ND TO ROOF |  | 3RD TO 6TH FLOOR |  |
|  |  |  |  | 7TH TO ROOF |  |

## Cont. Drafting

## Reinforcement Detailing of Pile Cap



## Cont. Drafting

## Pile Long Section Detail



## Planning of the Project

## Construction Scheduling



## Bill of Quantity(BOQ)

| Bill of Quantity |  |
| :---: | :---: |
| Items of Works | Amount (Taka) |
| Foundation | Approximately sixty three lakhs |
| Ground Floor | Approximately seventy four lakhs |
| Typical Floor | Approximately ninety five lakhs |
| Roof | Approximately twenty one lakhs |
| Shuttering | Approximately two lakhs |
| Rates of man, material and mark-ups | Approximately ten lakhs |

Total cost of this project will be approximately ten crores BDT.

## Conclusion

> Some of the features comply with the rules and others do not
> Main challenge was to finalize suitable beam, column dimensions that cater to serviceability and economy
> Software results checked by manual calculation and found reliable with 10\% error acceptable
> High construction cost due to poor accessibility to trucks
$>$ Estimated project span 11 months
> Estimated total cost 103129002.6 BDT

## Submission Files

Submission will contain-
> Report
> Logbook
> Meeting Minutes

THANK YOU

